

HYDROGEOLOGICAL INVESTIGATION REPORT PROPOSED RESIDENTIAL DEVELOPMENT 161 HEATHWOOD HEIGHTS DRIVE AURORA, ONTARIO L4G 4X2

Prepared for: Mr. Kamran Rzayev

161 Heathwood Heights Drive

Aurora, Ontario

L4G 4X2

Prepared by: Geomaple Geotechnics Inc.

60 Green Lane, Unit 12A

Thornhill, Ontario

L3T 7P5

Date: March 7, 2025

Project No.: 2024-10-150

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1 INTRODUCTION

Geomaple Geotechnics Inc. (Geomaple) was retained by Mr. Kamran Rzayev (the Client) to conduct a geotechnical investigation and hydrogeological study for the proposed residential development at 161 Heathwood Heights Drive, Aurora, Ontario.

The geotechnical investigation report was prepared and submitted to the client under a separate cover. The current report provides the results of the hydrogeological study at the site.

The subsurface investigation was completed by Geomaple which involved advancement of six (6) boreholes at the site, equipped with 50 mm monitoring wells, to depths ranging from about 5.0 to 12.2 m below ground surface.

Groundwater levels were observed in the boreholes upon completion of drilling and in all of the monitoring wells installed in the boreholes for 3 months following the completion of the drilling. Insitu permeability (rising/falling head) testing was conducted in selected monitoring wells to determine the hydraulic conductivity of the soil layers in which the wells were screened. Groundwater samples were taken from selected monitoring wells and tested as per City of Toronto Sewer By-laws for groundwater quality testing.

Based on the information obtained from this investigation, interpretation, analysis, and recommendations with respect to the hydrogeological engineering aspects of the proposed development were provided.

2 SITE AND PROJECT DESCRIPTION

The site is located in a residential neighbourhood and is bound by Heathwood Heights Drive to the north, Tilston Grove to the west, Aurora Walking/ Bike Trail to the south, and by residential properties to the east. The general location of the site is shown on Figure 1 in Appendix A.

The site currently contains an existing single-family house, which is proposed to be demolished to accommodate the redevelopment of the property. The redevelopment will involve severing the property into five (5) separate lots, with a new detached two-storey house constructed on each lot. The proposed site plan, prepared by Arcica Inc. dated January 31, 2025, was provided by the client.

3 PROCEDURE

The field investigation of the site was conducted on November 21, 2024 which consisted of drilling and sampling of six (6) boreholes at the site, five (5) boreholes to a depth of about 5 m and one (1) deep borehole to a depth of 12.2 m.

All boreholes were equipped with 50 mm diameter monitoring wells for groundwater level

measurements and in-situ hydraulic conductivity test. The results of the boreholes are recorded in detail on the Borehole Logs in Appendix B. The approximate borehole locations are shown on the enclosed Figure 2 in Appendix A.

The borehole surface elevations noted on the enclosed Borehole Logs are taken from the Survey drawing provided by the client. The borehole surface elevations are provided only for relating borehole soil stratigraphy and should not be used or relied on for other purposes.

The boring was drilled by specialist drilling subcontractor using a track mounted drill rig power auger with hollow stem augers/wash boring and were sampled with conventional 25 mm diameter split barrel samplers when the Standard Penetration Test (SPT) was carried out (ASTM D1586). The boreholes were equipped with a 50 mm diameter well for groundwater level monitoring. The field work (drilling, sampling, and testing) was observed and recorded by a member of our engineering staff, who logged the boring and examined the samples as they were obtained.

All samples obtained during the investigation were sealed into clean plastic bags and transported to our office for detailed inspection and testing. The samples were examined (tactile) in detail by our staff and classified according to visual and index properties.

The geotechnical laboratory testing consisted of water content determination as well as Sieve and Hydrometer Analysis on selected soil samples. The laboratory test results of individual samples are plotted on the Borehole Logs at respective sampling depth, and presented in Appendix C.

Groundwater levels were measured in the open boreholes upon completion of drilling and in the monitoring wells installed in the boreholes. The results of the groundwater level monitoring are summarized in Section 4.2 of this report.

In-situ permeability testing was carried out in selected monitoring wells to determine the hydraulic conductivity of the soil layers at which the wells were screened. Soil hydraulic conductivity values were also determined using grain size analysis as per empirical equations for sand soils. Results of the hydraulic conductivity testing based on in-situ testing and grain size analysis are summarized in Section 5 and presented in Appendix E.

4 SUBSURFACE CONDITIONS

The ground surface at the Site gradually slopes down towards southwest. No significant water bodies were located at the Site or within 250 m of the boundaries of the Site.

Borehole results indicated that underlying surficial layer of earth fill, the site is underlain by glacial till deposit, extending the full depth of investigation. Bedrock was not encountered in the boreholes and is expected to be at a significant depth below grade in the general area of the site.

Borehole/monitoring well locations are shown in Figure 2, in Appendix A. Details of the boreholes

are provided in the Borehole Logs in Appendix B. Summary of the subsurface condition is provided in the table below and the following sections.

4.1 Soil Stratigraphy

It should be noted that soil and groundwater conditions are confirmed at the borehole locations only and may vary at other locations. The stratigraphic boundaries as shown on the Borehole Logs represent an inferred transition between various strata, rather than a precise plane of geologic change.

4.1.1 Glacial Till

Below the topsoil, a layer of undisturbed native glacial till deposit was encountered extending to the full depth of investigation in all boreholes. The composition of the till varied from sandy silt to silty sand at the top, to clayey silt to silty clay at the middle, and silty sand at the lower portion of the glacial deposit.

The samples obtained from the till layer loose to very dense for the silty sand to sandy silt layers, and soft to very stiff for the clayey silt to silty clay layer representing Standard Penetration Test results ('N' Values) of 2 to 92 blows per 300 mm of penetration and 50 blows per 75 to 100 mm of penetration.

It should be noted that the glacial till the deposit is likely to contain larger particles (cobbles and boulders) that are not specifically identified in the borehole. The size and distribution of such obstructions cannot be predicted with borings, because the borehole sampler size is insufficient to secure representative samples for particles of this size.

4.2 Groundwater Level Monitoring

Groundwater levels were measured in the monitoring wells installed in the boreholes for a period of over 3 months. The following table summarized the results of groundwater level measurement:

Manitoring Data	Groundwater Depth (m)														
Monitoring Date	BH1	BH2	вн3	BH4	ВН5	вн6									
2024-11-29	Dry	Dry	Dry	Dry	6.60	Dry									
2024-12-13	Dry	Dry	Dry	Dry	6.78	Dry									
2024-12-27	Dry	Dry	Dry	Dry	6.41	Dry									
2025-01-09	Dry	Dry	Dry	Dry	5.30	Dry									
2025-01-31	2025-01-31 Dry		Dry	Dry	6.54	Dry									

Manitarina Data		Groundwater Depth (m)													
Monitoring Date	BH1	BH2	внз	BH4	ВН5	вн6									
2025-02-28	Dry	Dry	Dry	Dry	6.38	Dry									
Average	Dry	Dry	Dry	Dry	6.34	Dry									

Based on the results of groundwater level monitoring during a 3-month period, all boreholes except the deep borehole (BH5) were dry, while the average groundwater level depth in BH5 varied from 5.3 to 6.78 m, with an average of 6.34 m.

Monitoring Data	Groundwater Elevation (m)														
Monitoring Date	BH1	BH2	вн3	BH4	ВН5	ВН6									
2024-11-29	Dry	Dry	Dry	Dry	299.30	Dry									
2024-12-13	Dry	Dry	Dry	Dry	299.12	Dry									
2024-12-27	Dry	Dry	Dry	Dry	299.49	Dry									
2025-01-09	Dry	Dry	Dry	Dry	300.60	Dry									
2025-01-31	Dry	Dry	Dry	Dry	299.36	Dry									
2025-02-28	2025-02-28 Dry Average Dry		Dry	Dry	299.52	Dry									
Average			Dry	Dry	299.57	Dry									

Based on the results of groundwater level monitoring during a 3-month period, all boreholes except the deep borehole (BH5) were dry, while the average groundwater level elevation in BH5 varied from 299.12 to 300.6 m, with an average of 299.57 m.

5 SOIL HYDRAULIC CONDUCTIVITY

In-situ permeability tests (rising head and falling head tests) were carried out in three monitoring wells (BH3, BH4, and BH5) to estimate hydraulic conductivity (k) for the representative geological units at which the wells were screened. Measurements were recorded in addition to a data logger placed at the bottom of the well to monitor hydraulic head reduction. Hydraulic conductivity (k) values were calculated using "Bower & Rice" methods. The associated percolation time (min/cm) and infiltration rate (mm/hr) were also calculated for the non-saturated conditions (above the aquifer) in BH3 and BH4.

The following table presents the values of Hydraulic Conductivity, Percolation Time and Infiltration rate for the respective stratigraphic layer at which each well was screened.

BH No.	Screen Depth (m)	Soil Layer at Screen Level	Type of Test	Hydraulic Conductivity Ks (cm/s)	Percolation Time, T (min/cm)	Infiltration Rate, 1/T (mm/hr)
ВН3	3.0 – 4.5	Silty Sand Till	Falling Head	8.75 x 10 ⁻⁶	25	24
BH4	3.0 – 4.5	Silty Sand Till	Falling Head	3.1 x 10 ⁻⁵	18	34
BH5	9.1 – 12.1	Silty Sand Till	Rising Head	7.99 x 10 ⁻⁷	NA	NA

The hydraulic conductivity value for BH5 is based on the saturated conditions within the aquifer which can be used for calculation of groundwater volume within the excavation, while the Hydraulic Conductivity, Percolation Time, and Infiltration Rate values for BH3 and BH4 are based on a non-saturated conditions above the aquifer which can be used for the design of Low Impact Development (LID) facility at the site.

6 GRROUNDWTER QUALITY TESTING

Samples of groundwater were collected from BH5 and analysed for the York Region sewer use by-law parameters for sanitary and storm sewer.

The untreated sample exceeded the Limits for Storm Sewer Discharge for the following parameters:

- Total Suspended Solids
- Mn
- Toluene

The untreated sample did not exceed the Limits for Sanitary Sewer Discharge.

The untreated groundwater will require treatment prior to discharge into the storm and sanitary sewer systems.

The results of groundwater quality testing are presented in the Appendix D.

7 DISCUSSION AND ANALYSIS

7.1 Summary of Hydrogeological Conditions

The results of the investigation completed by Geomaple indicated the following hydrogeological features for the site:

- The site is underlain by a layer of silty sand to sandy silt till, underlain by a layer of clayey silt to silty clay till, overlying a layer of silty sand till.
- Bedrock was not encountered to the maximum depth of investigation and is expected to be at a significant depth.
- The average groundwater level elevation in the monitoring wells was at Elev. 299.57 m
- The hydraulic conductivity of the silty sand till in saturated condition is 7.99 x 10⁻⁷ cm/s.

7.2 Proposed Development Plan

Based on the information provided by the client the proposed development includes construction of five (5) detached houses with one level of basement. The followings are details of the excavation:

- The building size at each lot is approximately 11.89 m by 15.24 m.
- The excavation would roughly be rectangular in shape with a size of 12.9 m by 16.2 m and an approximate area of 209 m².
- As the underside of footing levels were not know at the time of preparation of this report, the maximum excavation depth was assumed to be 3.0 m below grade.
- The lowest existing grade elevation within the footprint of the proposed buildings is about 305.54 m.
- The deepest level of excavation would extend to Elev. 302.5 m which is about 3.0 m above the groundwater level at the site.

7.3 Groundwater Taking Volumes

7.3.1 During Excavation

As the excavation base is above the groundwater level, there would be no groundwater seepage expected within the excavation.

The above estimate does not consider stormwater from rainfall events. At account for the stormwater runoff on a rainy day during the construction at the Site, a 20 mm daily rainfall has been considered for the purpose of dewatering design. It is known that the excavation area is approximately 209 m². The total runoff volume for each lot is calculated by multiplying the excavation area by the rainfall intensity as follows:

Total Runoff Volume per lot = Excavation Area x Rainfall Intensity
=
$$209 \text{ m}^2 \text{ x } 0.02 \text{ m/day}$$

= $4.2 \text{ m}^3/\text{day}$

Based on the above, the total discharge volume from the excavation for short-term condition (during construction), considering the groundwater seepage and incidental precipitation, would be 4.2 m³/day.

The above-noted dewatering volume does not include a factor of safety. Considering a factor of safety of 1.2, the factored total dewatering volume during the excavation would be 5.0 m³/day.

7.3.2 After Construction (Foundation Drainage)

As the groundwater levels are below the proposed floor level, the perimeter foundation drainage system (if installed) would not collect any groundwater.

7.3.3 Summary of Discharge Volumes

A summary of discharge volumes during the excavation and after construction is presented in the following table.

	Discharge Volume (m³) – During Excavation												
Groundwat	er Seepage	Incidental P	recipitation	Total Di	scharge								
No FS	FS = 1.2	No FS	FS = 1.2	No FS	FS = 1.2								
0	0	4.2	5.0	4.2	5.0								

	Discharge Volume (m³) – After Construction (Foundation Drainage)													
Groundwat	ter Seepage	Incidental P	Precipitation	Total Di	scharge									
No FS	FS = 1.2	No FS	FS = 1.2	No FS	FS = 1.2									
0	0	0	0	0	0									

7.4 Assessment of Potential Impacts

7.4.1 Surface Water, Wetlands and Areas of Natural Significance

No water bodies were identified on the Site. Stormwater at the Site is expected to drain into landscaped areas on the Site or towards the catch basins located onsite or on the municipal roads adjacent to the Site.

There are no surface water bodies, wetlands, or other sensitive features on the Site or within the ZOI (groundwater) to be affected by potential dewatering activities.

7.4.2 Contamination Sources

The Site and immediately surrounding area currently consist mostly of residential. It is anticipated that the fill materials and native soils will be removed from the Site during redevelopment and therefore any impacts would be removed from the Site.

No environmental sampling and/or analysis of the soil or groundwater under O.Reg. 153/04 was conducted by Geomaple. As such, Geomaple cannot comment on the groundwater quality as it related to O.Reg. 153/04 for the Site.

8 LIMITATIONS AND USE OF REPORT

It must be recognized that there are special risks whenever engineering or related disciplines are applied to identify subsurface conditions. A comprehensive sampling and testing programme implemented in strict accordance with the most stringent level of care may fail to detect certain conditions. Geomaple has assumed for the purposes of providing advice, that the conditions that exist between sampling points are similar to those found at the sample locations. The conditions that Geomaple has interpreted to exist between sampling points can differ from those that exist. It must also be recognized that the passage of time, natural occurrences, and direct or indirect human intervention at or near the site have the potential to alter subsurface conditions.

The discussion and recommendations provided here are based on the factual data obtained from the investigation and are intended for use by the owner and its retained designers in the design phase of the project. Since the project is still in the design stage, all aspects of the project relative to the subsurface conditions cannot be anticipated. Geomaple should review the design drawings and specifications prior to the construction of this work. If there are changes to the project scope and development features, the interpretations made of the subsurface information, the geotechnical design parameters and comments relating to constructability issues and quality control may not be relevant to the revised project scope. Geomaple should be retained to review the implications of these changes with respect to the contents of this report.

The investigation at this site was conceived and executed to provide information for the slope stability study and the geotechnical design. It may not be possible to drill a sufficient number of boreholes, or samples and report them in a way that would provide all the subsurface information that could influence construction costs, techniques, equipment, and scheduling. Contractors bidding on or undertaking work on this project should therefore, in this light, be directed to decide on their own investigations, as well as their own interpretations of the factual investigation results. They should be cognizant of the risks implicit in subsurface investigation activities so that they may draw their own conclusions as to how the subsurface conditions may affect them.

This report was prepared for the express use of Mr. Kamran Rzayev and its retained design consultants. It is not for use by others. This report is copyright of Geomaple Geotechnics Inc., and no part of this report may be reproduced by any means, in any form, without the prior written permission of Geomaple Geotechnics Inc. and Mr. Kamran Rzayev., who are the authorized users.

It is recognized that the regulatory agencies in their capacities as the planning and building authorities under Provincial statutes, will make use of and rely upon this report, cognizant of the limitations thereof, both expressed and implied.

9 CLOSURE

We trust the foregoing information is sufficient for your present requirements. If you have any questions, or if we can be of further assistance, please do not hesitate to contact us.

Yours truly,

Geomaple Geotechnics Inc.

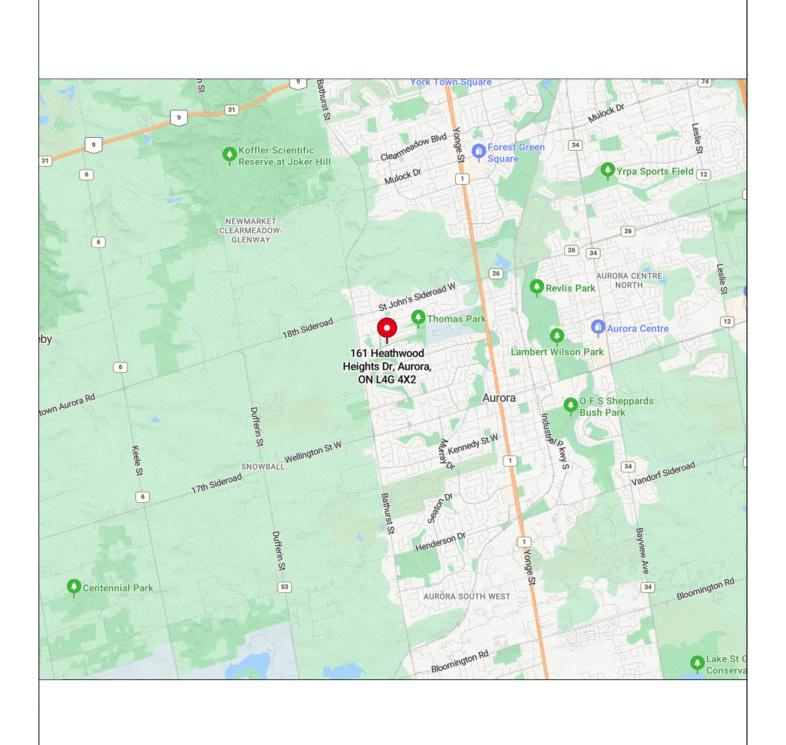


Navid Hatami, M.Eng, P.Eng. Senior Geotechnical Engineer

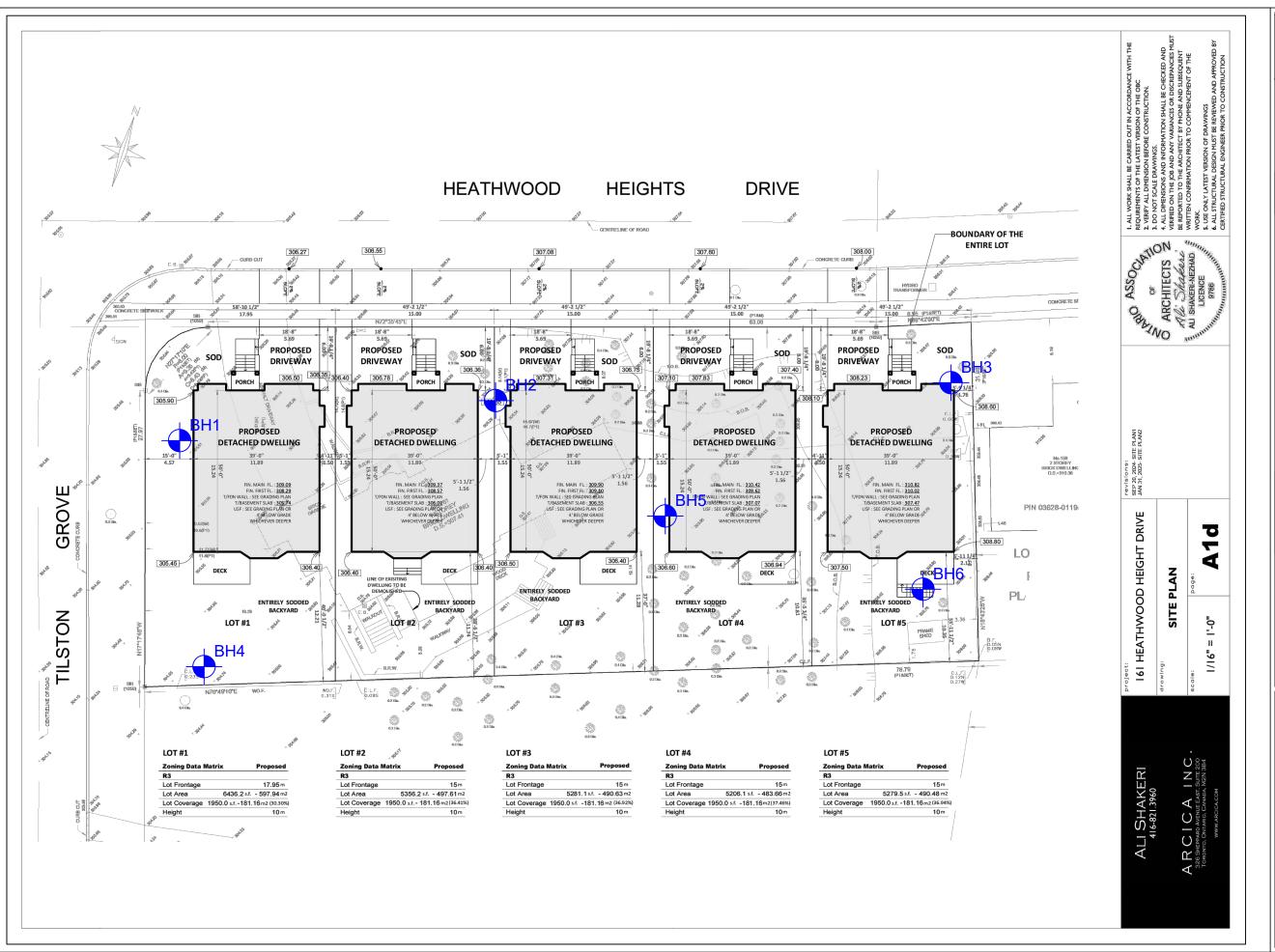
APPENDICES

APPENDIX A

FIGURES



	60 Green Lane, Unit 12A Thornhill, Ontario	No.	Revision	Date		161 Heathwood Aurora, 0	
	L3T 7P5					Site Locati	on Plan
*	Phone: (416) 444 1200				-	Project No.:	2024-10-150
	Fax: (416) 444 1200					Date:	March 2025
Geomaple Geotechnics Inc.	· · · · ·		+		_	Drawn by:	EL
					-	Checked by:	NH
						Figur	e 1
					-	Scale:	NA





Geomaple Geotechnics Inc.

60 Green Lane, Unit 12A Thornhill, Ontario L3T 7P5

Phone: (416) 444 1200 Fax: (416) 444 1200

Legend:



Borehole Location

No.	Description	Date

Project

161 Heathwood Heights Drive, Aurora, Ontario

Borehole Location Plan

Project Number 2024-10-150

Date March 2025

Drawn By EL

Checked By NH

Figure 2

le As Shown

APPENDIX B

BOREHOLE LOGS



PAGE 1 OF 1

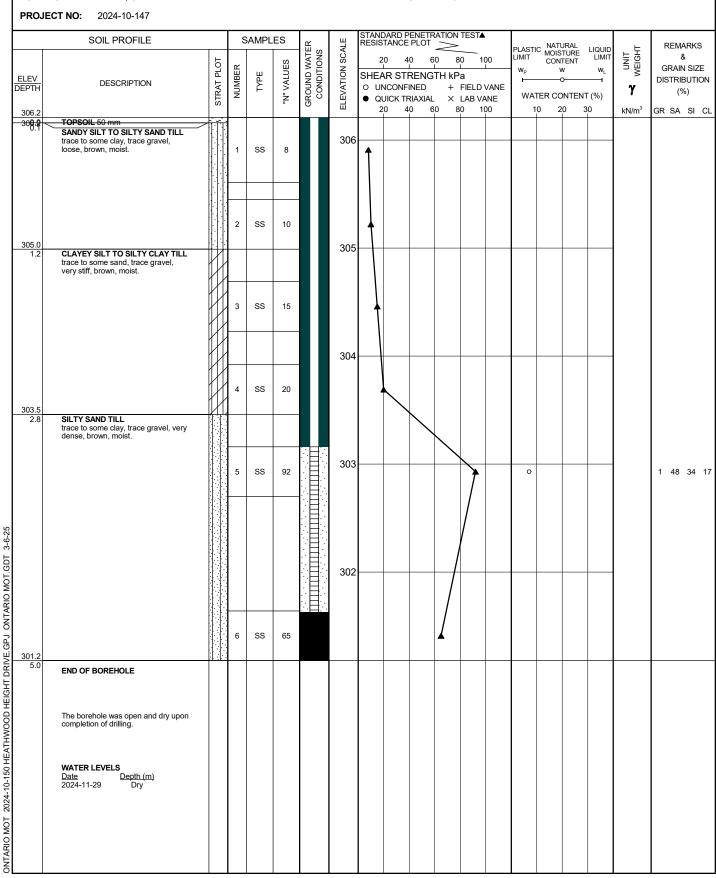
 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev

LOCAT	FION: Aurora, ON							DRI	LLIN	G DAT	E : 20	24-11	-21							
PROJE	ECT NO: 2024-10-147																			
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000	The borehole was open and dry upon completion of drilling.																			
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PAGE 1 OF 1

 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev





PAGE 1 OF 1

 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev

PROJ	ECT NO: 2024-10-147																		
	SOIL PROFILE		5	SAMPL	ES	ER C	ALE	STAN RESIS	DARD F	PENETE	RATION	TEST	`	PLAST	IC NAT	URAL	LIQUID	. 누	REMARKS
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PAGE 1 OF 1

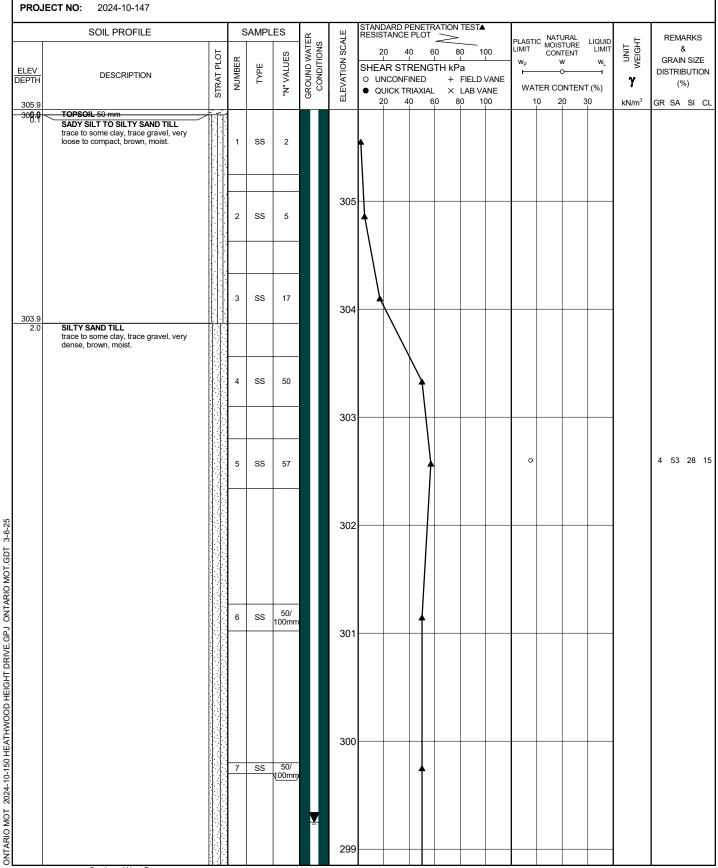
 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev

	CATION: Aurora, ON OJECT NO: 2024-10-147							DΚ	ILLIN	G DA	Γ E : 202	24-11-	-21							
	SOIL PROFILE		S	SAMPL	ES.	R	ALE.	STAND RESIST	ARD F	PENETE	RATION	TEST	\	PI AST	TIC NAT	URAL	LIQUID	E	REMA	ARKS
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302	2.2 SILTY SAND TILL trace to some clay, trace gravel, compact to very dense, brown, moist.						302													
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AI HWOOD HE	The borehole was open and dry upon completion of drilling.																			
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							2			refer to		20/								



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 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev





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 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev

LOCATION: Aurora, ON DRILLING DATE: 2024-11-21

	SOIL PROFILE		s	SAMPL	.ES	r	Щ	STANDARD P RESISTANCE	ENETRA PLOT	TION	TEST▲	Π	NAT	LIDAL			REMARKS
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES	GROUND WATER CONDITIONS	ELEVATION SCALE	20 4 SHEAR STF O UNCONFI O QUICK TF 20 4	RENGT NED	80 H kPa + 1	100 a FIELD VANE LAB VANE	W _P WA	TER CO	ITENT W O ONTENT	LIQUID LIMIT W _L T (%)	NNIT NOIL WEIGHT	& GRAIN SIZE DISTRIBUTION (%)
ДЕРТН	SILTY SAND TILL trace to some clay, trace gravel, very dense, brown, moist. (continued)	STRAT	8 9	SS	50/ \\75mm/		298 297 296	O UNCONFI	NED IAXIAL	+ F × L	FIELD VANE LAB VANE	WA					1
ONTARIO MOT 2024-10-150 HEATHWOOD HEIGHT DRIVE.GPJ ONTARIO MOT.GDT 3-6-25	END OF BOREHOLE The borehole was open and dry upon completion of drilling. WATER LEVELS Date Depth (m) 2024-11-29 6.60 m		11	SS	50/ \75mm/		294										



PAGE 1 OF 1

 PROJECT:
 161 Heathwood Heights Drive
 CLIENT:
 Kamran Rzayev

LOCA								DRILL	ING DA	TE: 202	24-11-2	21							
PROJI	SOIL PROFILE		5	SAMPL	FS	l	Ш	STANDAR RESISTAN	D PENET	RATION	TEST▲								
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES	GROUND WATER CONDITIONS	ELEVATION SCALE	20 SHEAR : O UNCO • QUICE	40 STRENC INFINED (TRIAXIA	60 8 STH kP +	0 10 a FIELD V LAB VA	/ANE	PLASTI LIMIT W _P 	TER CC	w O ONTEN	LIQUID LIMIT W _L T (%)	NNIT NEIGHT	REMA & GRAIN DISTRIB (% GR SA	SIZE UTION)
30 8.9	TOPSOIL 50 mm SANDY SILT TO SILTY SAND TILL trace to some clay, trace gravel, compact, brown, moist.		1	SS	10														
307.6 1.2	CLAYEY SILT TO SILTY CLAY TILL		2	SS	11		308												
1.2	trace to some sand, trace gravel, firm to stiff, brown, moist.		3	SS	15	-	307												
306.1	SILTY SAND TILL trace to some clay, trace gravel, compact to very dense, brown, moist.		4	SS	7		306	 											
	compact to very dense, brown, moisc		5	SS	17														
ARIO MOT.GDT 3-6-25							305												
303.8 303.8 5.0			6	SS	70		304						•					4 55	30 11
IGHT DR	END OF BOREHOLE																		
AI HWOOD HE	The borehole was open and dry upon completion of drilling.																		
ON TAKIO MOT 2024-10-150 HEATHWOOD HEIGHT DRIVE.GFJ ON TAKIO	WATER LEVELS Date Depth (m) 2024-11-29 Dry																		

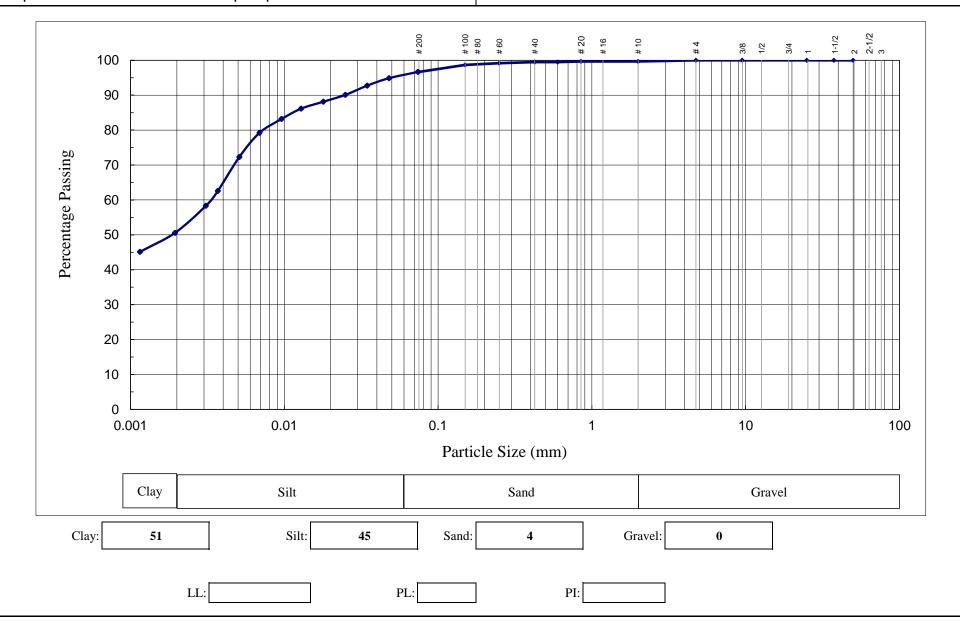
APPENDIX C

GEOTECHNICAL LABORATORY TEST RESULTS

GMGFR	-006-	R0
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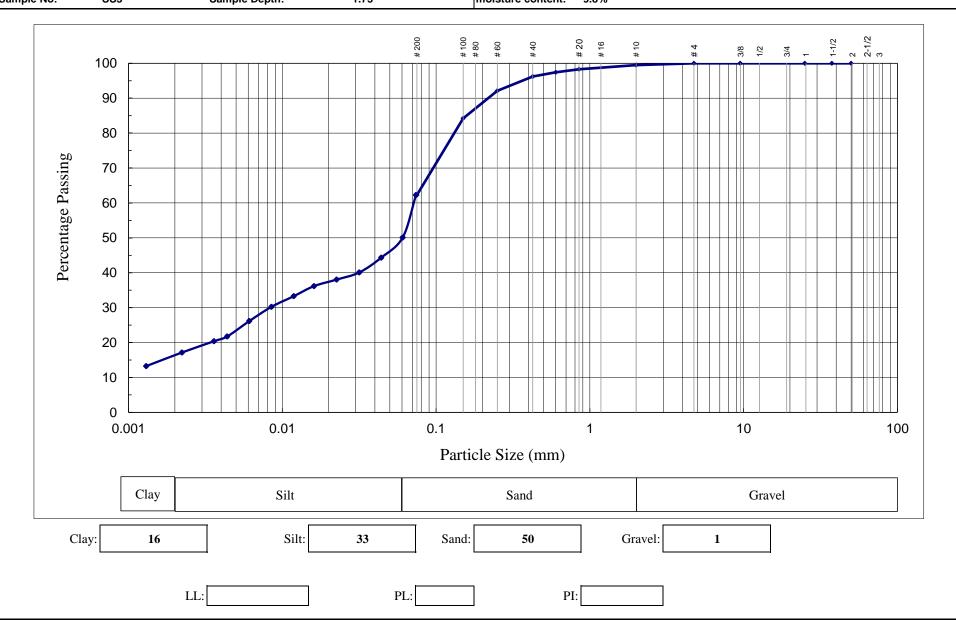
Project Name:	161 Heathwood He	ights Drive		Project No:	2024-10-150
Order No:		Test Date:	09-Dec-24	Client:	Kamran Rzayev
Borehole No:	BH1	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS4	Sample Depth:	2.52	moisture content:	30.8%



GMGFR-0)06-	R0
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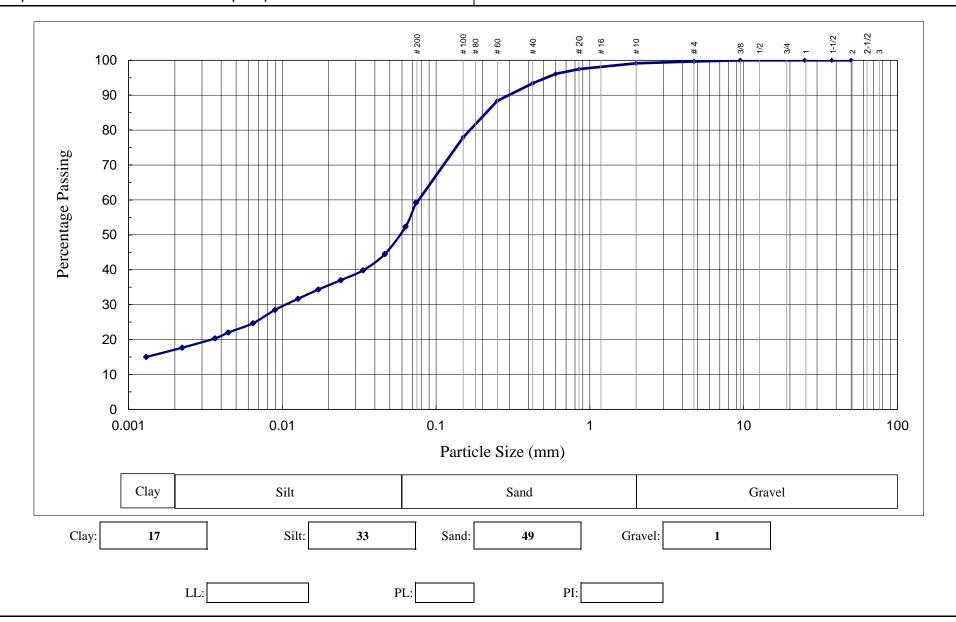
Project Name:	161 Heathwood He	eights Drive		Project No:	2024-10-150
Order No:		Test Date:	07-Dec-24	Client:	Kamran Rzayev
Borehole No:	BH2	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS3	Sample Depth:	1.75	moisture content:	9.8%



GMGFR-	-006-	R(
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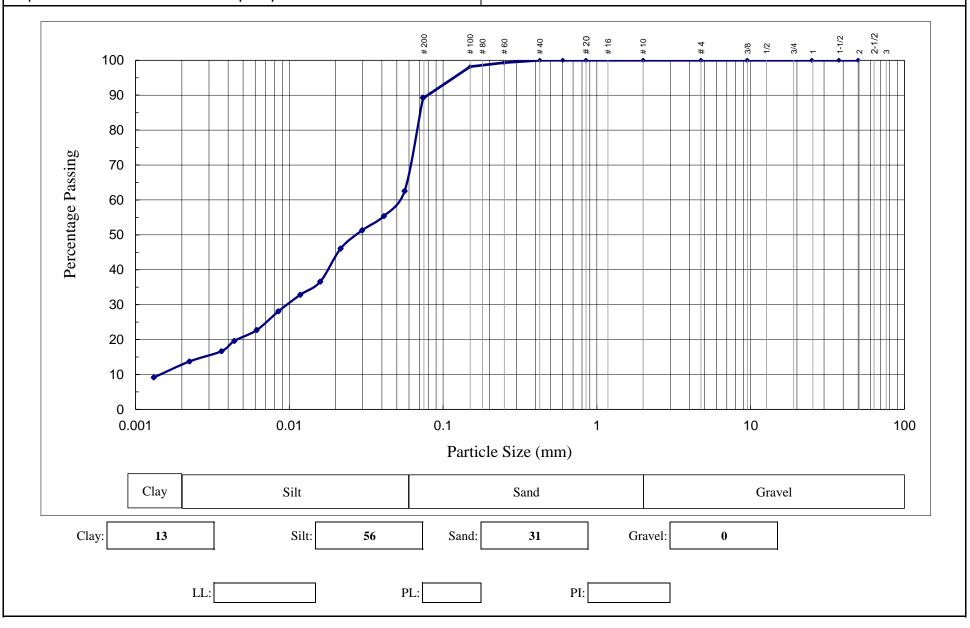
Project Name:	161 Heathwood	d Heights Drive		Project No:	2024-10-150
Order No:		Test Date:	09-Dec-24	Client:	Kamran Rzayev
Borehole No:	BH2	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS5	Sample Depth:	3.28	moisture content:	7.0%



GMGFR-0)06-	R0
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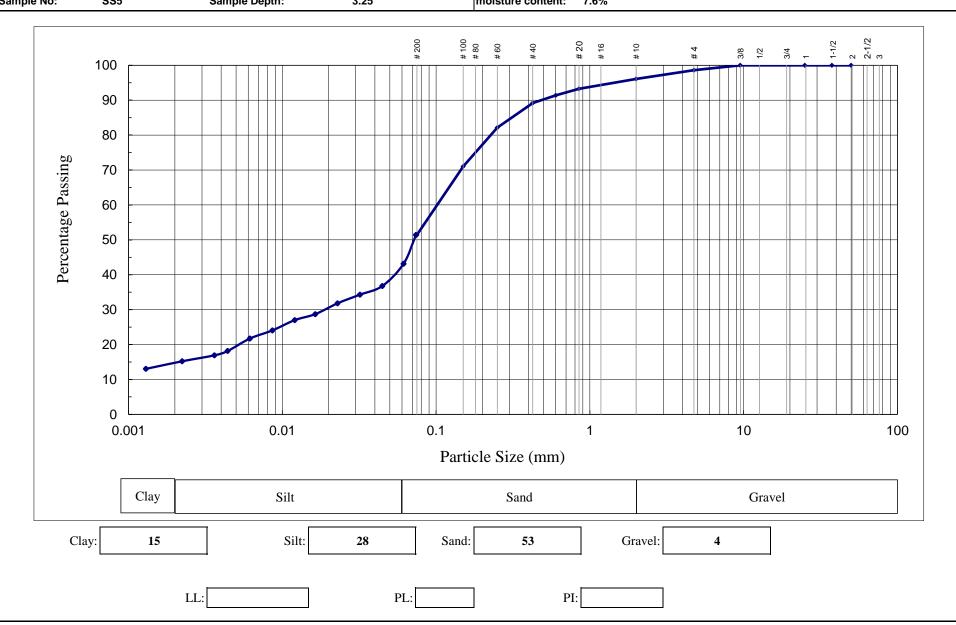
Project Name:	161 Heathwood He	eights Drive		Project No:	2024-10-150
Order No:		Test Date:	07-Dec-24	Client:	Kamran Rzayev
Borehole No:	BH4	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS3	Sample Depth:	1.75	moisture content:	20.3%



GMGFR-00	6-	к	(
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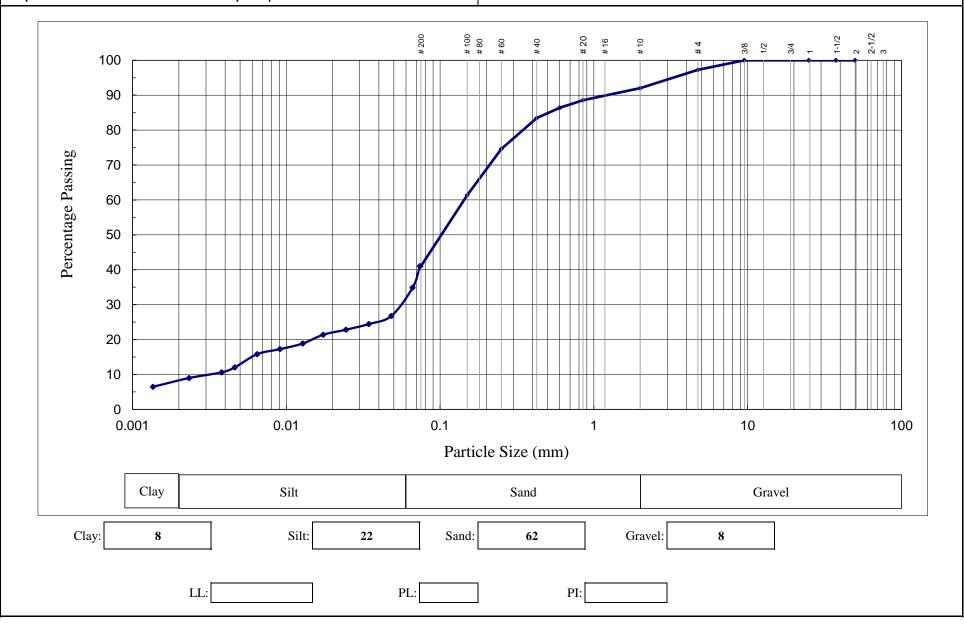
Project Name:	161 Heathwood Heights Drive			Project No:	2024-10-150
Order No:		Test Date:	07-Dec-24	Client:	Kamran Rzayev
Borehole No:	ВН5	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS5	Sample Depth:	3.25	moisture content:	7.6%



GMGFR-0)06-	R0
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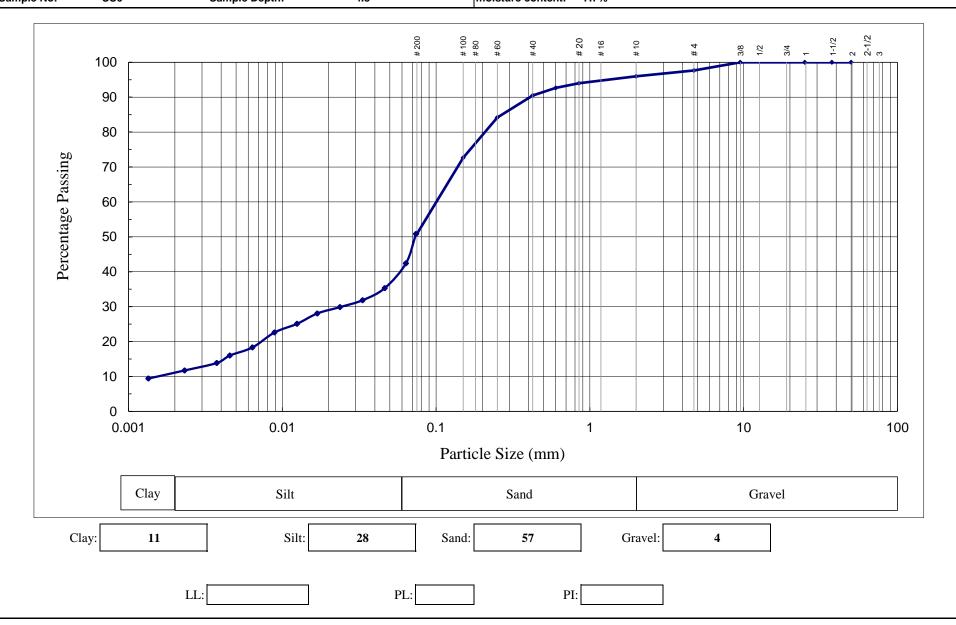
Project Name:	161 Heathwood Heights Drive			Project No:	2024-10-150
Order No:		Test Date:	06-Dec-24	Client:	Kamran Rzayev
Borehole No:	ВН5	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS10	Sample Depth:	10.73	moisture content:	11.5%



GMGFR-0)06-	R0
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Project Name:	161 Heathwood Heights Drive			Project No:	2024-10-150
Order No:		Test Date:	06-Dec-24	Client:	Kamran Rzayev
Borehole No:	вн6	Borehole Location:	Figure2	Lead Consultant:	
Sample No:	SS6	Sample Depth:	4.8	moisture content:	7.7%



APPENDIX D

ANALYTICAL TEST RESULTS



Certificate of Analysis

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319 Date Submitted: 2024-12-16 Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

Dear	Na	vid:

Please find attached the analytical results for your samples. If you have any questions regarding this report, please do not hesitate to call (613-727-5692).

Page 1 of 9

Report Comments:

APPROVAL:	
	Patrick Jacques, Chemist

All analysis is completed at Eurofins Environment Testing Canada Inc. (Ottawa, Ontario) unless otherwise indicated.

Eurofins Environment Testing Canada Inc. (Ottawa, Ontario) is accredited by CALA, Canadian Association for Laboratory Accreditation to ISO/IEC 17025 for tests which appear on the scope of accreditation. The scope is available at: https://directory.cala.ca/.

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Eurofins Environment Testing Canada Inc. (Ottawa, Ontario) is accredited by the Ontario Ministry of Agriculture, Food, and Rural Affairs for specific tests in agricultural soils.

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Eurofins_multisample(L)44.rpt

Certificate of Analysis



Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

				Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D.	1754409 WW 2024-12-16 2024-10-150-WS1
Group	Analyte	MRL	Units	Guideline	
Anions	F	0.10	mg/L		0.14
	SO4	1	mg/L		48
General Chemistry	BOD5	1	mg/L	MAC 15	<1
	Cyanide (total)	0.005	mg/L	MAC 0.020	<0.005
	рН	1.00		6.0-9.0	7.83
	Phenols	0.001	mg/L	MAC 0.008	0.003
	Total Suspended Solids	2	mg/L	MAC 15	170*
Mercury	Hg	0.0001	mg/L	MAC 0.0004	<0.0001
Metals	Ag	0.01	mg/L	MAC 0.120	<0.01
	Al	0.1	mg/L		1.1
	Aqua-Regia Digest				у
	As	0.02	mg/L	MAC 0.020	<0.02
	Cd	0.008	mg/L	MAC 0.008	<0.008
	Со	0.01	mg/L		<0.01
	Cr	0.05	mg/L	MAC 0.080	<0.05
	Cu	0.01	mg/L	MAC 0.050	<0.01
	Mn	0.01	mg/L	MAC 0.150	0.23*
	Мо	0.01	mg/L		<0.01
	Ni	0.01	mg/L	MAC 0.080	<0.01
	Pb	0.01	mg/L	MAC 0.120	<0.01
	Sb	0.01	mg/L		<0.01
	Se	0.02	mg/L	MAC 0.020	<0.02
	Sn	0.1	mg/L		<0.1
	Ti	0.1	mg/L		<0.1
	Zn	0.04	mg/L	MAC 0.040	<0.04

Guideline = Storm Sewer - York

* = Guideline Exceedence

Results relate only to the parameters tested on the samples submitted. Methods references and/or additional QA/QC information available on request.

MRL = Method Reporting Limit, AO = Aesthetic Objective, OG = Operational Guideline, MAC = Maximum Acceptable Concentration, IMAC = Interim Maximum Acceptable Concentration, STD = Standard, PWQO = Provincial Water Quality Guideline, IPWQO = Interim Provincial Water Quality Objective, TDR = Typical Desired Range

Certificate of Analysis



Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

				Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D.	1754409 WW 2024-12-16 2024-10-150-WS1
Group	Analyte	MRL	Units	Guideline	
Nutrients	Total Kjeldahl Nitrogen	0.100	mg/L	MAC 1	0.674
	Total P	0.020	mg/L	MAC 0.400	0.110
Oil and Grease	Oil & Grease - Mineral	1	mg/L		<1
	Oil & Grease - Non-mineral	1	mg/L		<1
	Oil & Grease - Total	1	mg/L		<1
Others	Nonylphenol	0.20	ug/L		<0.20
	Nonylphenol Ethoxylates (Total)	0.10	ug/L		0.14
PCBs	Polychlorinated Biphenyls (PCBs)	0.1	ug/L	MAC 0.4	<0.1
Semi-Volatiles	Bis(2-ethylhexyl)phthalate	0.4	ug/L	MAC 8.8	0.7
	Di-n-butylphthalate	1.3	ug/L	MAC 15.0	<1.3
VOCs Surrogates	1,2-dichloroethane-d4	0	%		104
	4-bromofluorobenzene	0	%		98
	Toluene-d8	0	%		112
Volatiles	1,1,2,2-tetrachloroethane	0.5	ug/L	MAC 17.0	<0.5
	1,2-dichlorobenzene	0.4	ug/L	MAC 5.6	<0.4
	1,4-dichlorobenzene	0.4	ug/L	MAC 6.8	<0.4
	Benzene	0.5	ug/L	MAC 2.0	<0.5
	c-1,2-Dichloroethylene	0.4	ug/L	MAC 5.6	<0.4
	Chloroform	0.5	ug/L	MAC 2.0	<0.5
	Dichloromethane	4.0	ug/L	MAC 5.2	<4.0
	Ethylbenzene	0.5	ug/L	MAC 2.0	<0.5
	m/p-xylene	0.4	ug/L		0.7
	Methyl Ethyl Ketone (MEK)	2	ug/L		<2
	o-xylene	0.4	ug/L		<0.4
	Styrene	0.5	ug/L		<0.5

Guideline = Storm Sewer - York

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Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

Group	Analyte	MRL	Units	Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. Guideline	1754409 WW 2024-12-16 2024-10-150-WS1
Volatiles	t-1,3-Dichloropropylene	0.5	ug/L	MAC 5.6	<0.5
Volatiles	Tetrachloroethylene	0.3		MAC 4.4	<0.3
			ug/L		
	Toluene	0.4	ug/L	MAC 2.0	2.1*
	Trichloroethylene	0.3	ug/L	MAC 8.0	<0.3
	Xylene; total	0.5	ug/L	MAC 4.4	0.7

Guideline = Storm Sewer - York

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Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank		QC % Rec	QC Limits
Run No 469919 Analysis/Extraction Date 20 Method B 625/P 8270)24-12-18 A n	alyst	СМ	
Bis(2-ethylhexyl)phthalate	<0.4 ug/L		91	20-140
Di-n-butylphthalate	<1.3 ug/L		93	20-140
Run No 470224 Analysis/Extraction Date 20 Method C SM2540)24-12-18 A n	alyst	SKH	
Total Suspended Solids	<2 mg/L		97	90-110
Run No 470293 Analysis/Extraction Date 20 Method SM2320,2510,4500H/F)24-12-18 A n	alyst	AsA	
F	<0.10 mg/L		102	90-110
рН			101	90-110
Run No 470297 Analysis/Extraction Date 20 Method SM 4110)24-12-19 A n	alyst	IP	
SO4	<1 mg/L		110	90-110
Run No 470298 Analysis/Extraction Date 20 Method SM 5210B)24-12-23 A n	alyst	z s	
BOD5	<1 mg/L		90	75-125
Run No 470322 Analysis/Extraction Date 20 Method SM5530D/EPA420.2)24-12-19 A n	alyst	IP	

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Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Phenols	<0.001 mg/L	105	50-120
Run No 470326 Analysis/Extraction Date 20 Method EPA 8081B)24-12-19 Ana	alyst DT	
Polychlorinated Biphenyls	<0.1 ug/L	102	60-140
Run No 470328 Analysis/Extraction Date 20 Method EPA 351.2	024-12-19 Ana	alyst SKH	
Total Kjeldahl Nitrogen	<0.100 mg/L	99	70-130
Run No 470329 Analysis/Extraction Date 20 Method EPA 200.8)24-12-19 Ana	alyst AaN	
Silver	<0.01 mg/L	110	70-130
Aluminum	<0.1 mg/L	108	70-130
Aqua-Regia Digest			
Arsenic	<0.02 mg/L	102	70-130
Cadmium	<0.008 mg/L	108	70-130
Cobalt	<0.01 mg/L	110	70-130
Chromium Total	<0.05 mg/L	111	70-130
Copper	<0.01 mg/L	115	70-130
Manganese	<0.01 mg/L	111	70-130
Molybdenum	<0.01 mg/L	96	70-130

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Environment Testing

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156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits		
Nickel	<0.01 mg/L	112	70-130		
Lead	<0.01 mg/L	105	70-130		
Antimony	<0.01 mg/L	109	70-130		
Selenium	<0.02 mg/L	110	70-130		
Sn	<0.1 mg/L	88	70-130		
Titanium	<0.1 mg/L	93	70-130		
Zinc	<0.04 mg/L	106	70-130		
Run No 470350 Analysis/Extraction Date 2024-12-19 Analyst AaN Method M SM3112B-3500B					
Mercury	<0.0001 mg/L	92	76-123		
Run No 470355 Analysis/Extraction Date 20 Method EPA 8260)24-12-18 Ana	alyst HS			
Tetrachloroethane, 1,1,2,2-	<0.5 ug/L	85	60-130		
Dichlorobenzene, 1,2-	<0.4 ug/L	98	60-130		
Dichlorobenzene, 1,4-	<0.4 ug/L	102	60-130		
Benzene	<0.5 ug/L	99	60-130		
Dichloroethylene, 1,2-cis-	<0.4 ug/L	99	60-130		
Chloroform	<0.5 ug/L	112	60-130		

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Environment Testing

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156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

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Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Methylene Chloride	<4.0 ug/L	97	60-130
Ethylbenzene	<0.5 ug/L	102	60-130
m/p-xylene	<0.4 ug/L	102	60-130
Methyl Ethyl Ketone	<2 ug/L	110	60-130
o-xylene	<0.4 ug/L	92	60-130
Styrene	<0.5 ug/L	103	60-130
Dichloropropene,1,3-trans-	<0.5 ug/L	91	60-130
Tetrachloroethylene	<0.3 ug/L	127	60-130
Toluene	<0.4 ug/L	104	60-130
Trichloroethylene	<0.3 ug/L	99	60-130
Run No 470385 Analysis/Extraction Date 20 Method EPA 365.1)24-12-20 Ana	ilyst SKH	
Total P	<0.020 mg/L	102	80-120
Run No 470413 Analysis/Extraction Date 20 Method EPA 8260	024-12-20 A na	ulyst HS	
Xylene Mixture			
Run No 470428 Analysis/Extraction Date 20 Method SM 5520B/F)24-12-23 A na	ilyst ACN	
Oil & Grease - Mineral	<1 mg/L	80	70-130

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Environment Testing

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156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

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Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Oil & Grease - Non-mineral	<1 mg/L		70-130
Oil & Grease - Total	<1 mg/L	95	70-130
Run No 470443 Analysis/Extraction Date 20 Method SM4500-CNC/MOE E3015 Cyanide (total)	24-12-23 Ana	alyst Z S 88	61-139
Run No 470448 Analysis/Extraction Date 20 Method ASTM D7485	024-12-20 A na	il yst SD	
Nonylphenol	<0.20 ug/L	96	50-150
Nonylphenol Ethoxylates (Total)			

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Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

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Report Number: 3013319
Date Submitted: 2024-12-16
Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

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Page 1 of 9

Report Comments:

APPROVAL:	
	Patrick Jacques, Chemist

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Eurofins_multisample(L)44.rpt



Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

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Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

Group	Analyte	MRL	Units	Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. Guideline	1754409 WW 2024-12-16 2024-10-150-WS1
Anions	F	0.10	mg/L	MAC 10	0.14
Allions	SO4	1	mg/L	MAC 1500	48
General Chemistry	BOD5	1	mg/L	MAC 300	<1
Octional Officialistry	Cyanide (total)	0.005	mg/L	MAC 2	<0.005
	pH	1.00	1119/1	6.0-10.5	7.83
	Phenols	0.001	mg/L	MAC 1.0	0.003
	Total Suspended Solids	2	mg/L	MAC 350	170
Mercury	Hg	0.0001	mg/L	MAC 0.01	<0.0001
Metals	Ag	0.01	mg/L	MAC 5	<0.01
	AI	0.1	mg/L	MAC 50	1.1
	Aqua-Regia Digest				У
	As	0.02	mg/L	MAC 1	<0.02
	Cd	0.008	mg/L	MAC 0.7	<0.008
	Со	0.01	mg/L	MAC 5	<0.01
	Cr	0.05	mg/L	MAC 2	<0.05
	Cu	0.01	mg/L	MAC 3	<0.01
	Mn	0.01	mg/L	MAC 5	0.23
	Мо	0.01	mg/L	MAC 5	<0.01
	Ni	0.01	mg/L	MAC 2	<0.01
	Pb	0.01	mg/L	MAC 1	<0.01
	Sb	0.01	mg/L	MAC 5	<0.01
	Se	0.02	mg/L	MAC 1	<0.02
	Sn	0.1	mg/L	MAC 5	<0.1
	Ti	0.1	mg/L	MAC 5	<0.1
	Zn	0.04	mg/L	MAC 2	<0.04

Guideline = Sanitary Sewer - York

* = Guideline Exceedence

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Environment Testing

Client: Geomaple Geotechnics

156 St. Regis Cres. S. North York, Ontario

M3J 1Y8

Attention: Mr. Navid

PO#:

Invoice to: Geomaple Geotechnics

Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

				Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D.	1754409 WW 2024-12-16 2024-10-150-WS1
Group	Analyte	MRL	Units	Guideline	
Nutrients	Total Kjeldahl Nitrogen	0.100	mg/L	MAC 100	0.674
	Total P	0.020	mg/L	MAC 10	0.110
Oil and Grease	Oil & Grease - Mineral	1	mg/L	MAC 15	<1
	Oil & Grease - Non-mineral	1	mg/L	MAC 150	<1
	Oil & Grease - Total	1	mg/L		<1
Others	Nonylphenol	0.20	ug/L	MAC 20	<0.20
	Nonylphenol Ethoxylates (Total)	0.10	ug/L	MAC 200	0.14
PCBs	Polychlorinated Biphenyls (PCBs)	0.1	ug/L	MAC 1	<0.1
Semi-Volatiles	Bis(2-ethylhexyl)phthalate	0.4	ug/L	MAC 12	0.7
	Di-n-butylphthalate	1.3	ug/L	MAC 80	<1.3
VOCs Surrogates	1,2-dichloroethane-d4	0	%		104
	4-bromofluorobenzene	0	%		98
	Toluene-d8	0	%		112
Volatiles	1,1,2,2-tetrachloroethane	0.5	ug/L	MAC 1400	<0.5
	1,2-dichlorobenzene	0.4	ug/L	MAC 50	<0.4
	1,4-dichlorobenzene	0.4	ug/L	MAC 80	<0.4
	Benzene	0.5	ug/L	MAC 10	<0.5
	c-1,2-Dichloroethylene	0.4	ug/L	MAC 4000	<0.4
	Chloroform	0.5	ug/L	MAC 40	<0.5
	Dichloromethane	4.0	ug/L	MAC 2000	<4.0
	Ethylbenzene	0.5	ug/L	MAC 160	<0.5
	m/p-xylene	0.4	ug/L		0.7
	Methyl Ethyl Ketone (MEK)	2	ug/L	MAC 8000	<2
	o-xylene	0.4	ug/L		<0.4
	Styrene	0.5	ug/L	MAC 200	<0.5

Guideline = Sanitary Sewer - York

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Project: 161 Heathwood Heights Drive

COC #: 232064

				Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D.	1754409 WW 2024-12-16 2024-10-150-WS1
Group	Analyte	MRL	Units	Guideline	
Volatiles	t-1,3-Dichloropropylene	0.5	ug/L	MAC 140	<0.5
	Tetrachloroethylene	0.3	ug/L	MAC 1000	<0.3
	Toluene	0.4	ug/L	MAC 270	2.1
	Trichloroethylene	0.3	ug/L	MAC 400	<0.3
	Xylene; total	0.5	ug/L	MAC 1400	0.7

Guideline = Sanitary Sewer - York

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Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank		QC % Rec	QC Limits
Run No 469919 Analysis/Extraction Date 20 Method B 625/P 8270)24-12-18 A n	alyst	СМ	
Bis(2-ethylhexyl)phthalate	<0.4 ug/L		91	20-140
Di-n-butylphthalate	<1.3 ug/L		93	20-140
Run No 470224 Analysis/Extraction Date 20 Method C SM2540)24-12-18 A n	alyst	SKH	
Total Suspended Solids	<2 mg/L		97	90-110
Run No 470293 Analysis/Extraction Date 20 Method SM2320,2510,4500H/F)24-12-18 A n	alyst	AsA	
F	<0.10 mg/L		102	90-110
рН			101	90-110
Run No 470297 Analysis/Extraction Date 20 Method SM 4110)24-12-19 A n	alyst	IP	
SO4	<1 mg/L		110	90-110
Run No 470298 Analysis/Extraction Date 20 Method SM 5210B)24-12-23 A n	alyst	z s	
BOD5	<1 mg/L		90	75-125
Run No 470322 Analysis/Extraction Date 20 Method SM5530D/EPA420.2)24-12-19 A n	alyst	IP	

Guideline = Sanitary Sewer - York

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Report Number: 3013319

Date Submitted: 2024-12-16

Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Phenols	<0.001 mg/L	105	50-120
Run No 470326 Analysis/Extraction Date 20 Method EPA 8081B)24-12-19 Ana	alyst DT	
Polychlorinated Biphenyls	<0.1 ug/L	102	60-140
Run No 470328 Analysis/Extraction Date 20 Method EPA 351.2	024-12-19 Ana	alyst SKH	
Total Kjeldahl Nitrogen	<0.100 mg/L	99	70-130
Run No 470329 Analysis/Extraction Date 20 Method EPA 200.8)24-12-19 Ana	alyst AaN	
Silver	<0.01 mg/L	110	70-130
Aluminum	<0.1 mg/L	108	70-130
Aqua-Regia Digest			
Arsenic	<0.02 mg/L	102	70-130
Cadmium	<0.008 mg/L 108		70-130
Cobalt	<0.01 mg/L 110		70-130
Chromium Total	<0.05 mg/L	111	70-130
Copper	<0.01 mg/L	115	70-130
Manganese	<0.01 mg/L	111	70-130
Molybdenum	<0.01 mg/L	96	70-130

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COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Nickel	<0.01 mg/L	112	70-130
Lead	<0.01 mg/L	105	70-130
Antimony	<0.01 mg/L	109	70-130
Selenium	<0.02 mg/L	110	70-130
Sn	<0.1 mg/L	88	70-130
Titanium	<0.1 mg/L	93	70-130
Zinc	<0.04 mg/L	106	70-130
Run No 470350 Analysis/Extraction Date 20 Method M SM3112B-3500B)24-12-19 A na	alyst AaN	
Mercury	<0.0001 mg/L	92	76-123
Run No 470355 Analysis/Extraction Date 20 Method EPA 8260)24-12-18 Ana	alyst HS	
Tetrachloroethane, 1,1,2,2-	<0.5 ug/L	85	60-130
Dichlorobenzene, 1,2-	<0.4 ug/L	98	60-130
Dichlorobenzene, 1,4-	<0.4 ug/L	102	60-130
Benzene	<0.5 ug/L	99	60-130
Dichloroethylene, 1,2-cis-	<0.4 ug/L	99	60-130
Chloroform	<0.5 ug/L	112	60-130

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Date Submitted: 2024-12-16
Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Methylene Chloride	<4.0 ug/L	97	60-130
Ethylbenzene	<0.5 ug/L	102	60-130
m/p-xylene	<0.4 ug/L	102	60-130
Methyl Ethyl Ketone	<2 ug/L	110	60-130
o-xylene	<0.4 ug/L	92	60-130
Styrene	<0.5 ug/L	103	60-130
Dichloropropene,1,3-trans-	<0.5 ug/L	91	60-130
Tetrachloroethylene	<0.3 ug/L 127		60-130
Toluene	<0.4 ug/L	104	60-130
Trichloroethylene	<0.3 ug/L	99	60-130
Run No 470385 Analysis/Extraction Date 20 Method EPA 365.1)24-12-20 Ana	ilyst SKH	
Total P	<0.020 mg/L	102	80-120
Run No 470413 Analysis/Extraction Date 20 Method EPA 8260	024-12-20 A na	ulyst HS	
Xylene Mixture			
Run No 470428 Analysis/Extraction Date 20 Method SM 5520B/F)24-12-23 A na	ilyst ACN	
Oil & Grease - Mineral	<1 mg/L	80	70-130

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Date Reported: 2024-12-23

Project: 161 Heathwood Heights Drive

COC #: 232064

QC Summary

Analyte	Blank	QC % Rec	QC Limits
Oil & Grease - Non-mineral	<1 mg/L		70-130
Oil & Grease - Total	<1 mg/L	95	70-130
Run No 470443 Analysis/Extraction Date 20 Method SM4500-CNC/MOE E3015	24-12-23 Ana	alyst Z S	61-139
Cyanide (total) Run No 470448 Analysis/Extraction Date 20 Method ASTM D7485	ı	llyst SD	01-139
Nonylphenol	<0.20 ug/L	96	50-150
Nonylphenol Ethoxylates (Total)			

Guideline = Sanitary Sewer - York

* = Guideline Exceedence

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APPENDIX E

IN-SITU PERMEABILITY TEST RESULTS



Well Response Test

Project: 161 Heathwood Heights Drive

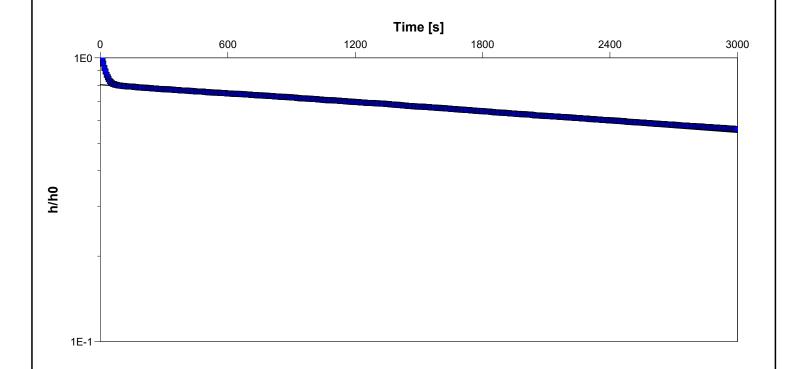
Number: 2024-10-150

Client:

Kamran Rzayev Test Well: BH3

Test Conducted by: Ehsan Lari Test Date: 2024-12-13

Falling Head Analysis Performed by: Ehsan Lari Analysis Date: 2024-12-17



Observation Well	Hydraulic Conductivity	
	[m/s]	
ВН3	8.75 × 10 ⁻⁸	

*Low Impact Development Stormwater Management Planning and Design Guide Appendix C (Version 1.0, 2010) TRCA & CVC

Infiltration Rate (mm/hr)	Percolation Time (minute/cm)	*Hydraulic Conductivity, Kfs (cm/s)
24	25	8.75E-06



Well Response Test

Project: 161 Heathwood Heights Drive

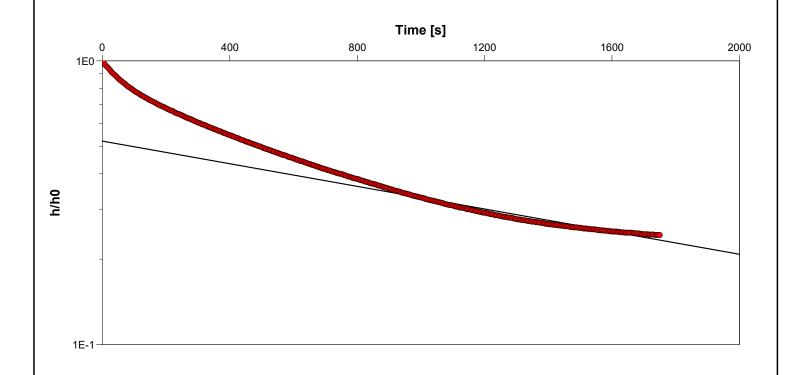
Number: 2024-10-150

Client: Kamran Rzayev

Test Well: BH4

Test Conducted by: Ehsan Lari Test Date: 2024-12-13

Analysis Performed by: Ehsan Lari Falling Head Analysis Date: 2024-12-17



Observation Well	Hydraulic Conductivity	
	[m/s]	
BH4	3.10 × 10 ⁻⁷	

*Low Impact Development Stormwater Management Planning and Design Guide Appendix C (Version 1.0, 2010) TRCA & CVC

Infiltration Rate (mm/hr)	Percolation Time (minute/cm)	*Hydraulic Conductivity, Kfs (cm/s)
34	18	3.10E-05



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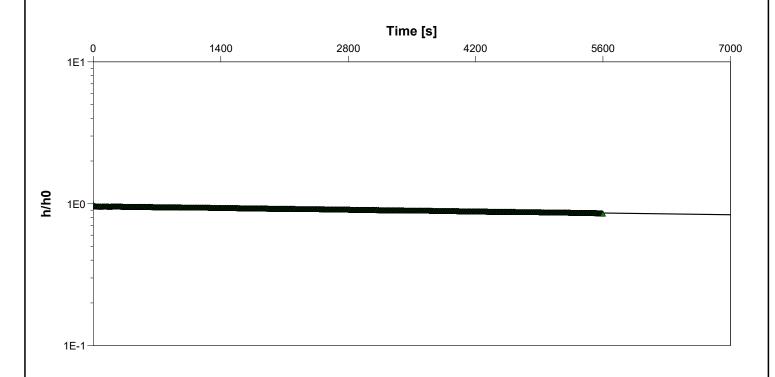
Project: 161 Heathwood Heights Drive

Number: 2024-10-150

Client: Kamran Rzayev

Location: Aurora, Ontario	Slug Test: Well Response BH5	Test Well: BH5
Test Conducted by: Ehsan Lari		Test Date: 2024-12-13
Analysis Performed by: Ehsan Lari	Rising Head	Analysis Date: 2024-12-17

Aquifer Thickness: 6.00 m



Calculation using Bouwer & Rice

Observation Well	Hydraulic Conductivity	
	[m/s]	
BH5	7.99 × 10 ⁻⁹	

